



Estimation of Runoff Potential using Curve Number Method

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Abstract: Runoff estimation is important for hydrological structure design. The SCS curve number (SCS-CN) approach is a popular technique for assessing rainfall-induced direct runoff. In this study, the runoff depth was calculated for the Hebburu micro-watershed in Tarikere Taluk, Chikkamagaluru district, Karnataka, using the Soil Conservation Service-Curve Number (SCS-CN) method. The Hebbur Micro-Watershed, which spans an area of 1038 ha, is situated between latitudes of 13° 46' 14.81" N to 13° 44' 23.95" N and longitudes of 75° 58' 24.38" E to 76° 0' 41.14" E. For the analysis, many parameters including information on the soil, rainfall, and land use were taken into account. Daily runoff was estimated between 2009-2018. Highest runoff was observed during 2014 (520 mm) followed by 2010 and 2009. During 2016 lowest runoff was generated in the watershed due to low rainfall in the year. Correlation coefficient between rainfall and runoff was found 97 per cent between 2009-2018. The results showed that the curve number approach, when combined with remote sensing and GIS technologies, can be effectively be used to estimate the runoff in an ungauged watershed.

Keywords: Curve number, Runoff potential, Watershed, Remote sensing, Soil conservation service

A watershed is a region that drains to a single outlet. For proper watershed management, such as flood control and its management, irrigation and drainage network design, hydropower generation, etc., accurate runoff depth and volume estimation is an essential responsibility. Rainfall-generated runoff is influenced by many factors, including soil type, vegetation, and types of land use, in addition to the intensity, length, and distribution of rainfall. The current study uses the Natural Resource Conservation Service Curve Number (NRCS-CN) method to calculate the runoff depth (Rajbanshi 2016). There are various methods for modelling rainfall and runoff, including the Artificial Neural Network (ANN), the SCS Curve number model, the hydrograph, and others. (Askar 2014). SCS-CN technique is one of the simplest methods for rainfall runoff modelling. The curve number is based on the area's of hydrologic soil group, land use, treatment and hydrological conditions (Askar 2014). There are four hydrologic soil groups: A, B, C and D. Group A have high infiltration rates and group D have low infiltration rates (Sahoo and Patra 2014). The Soil Conservation Service Curve Number (SCS-CN) method is widely used for predicting direct runoff volume for a given rainfall event (Soulis and Valiantzas 2012). The objective of the study was to estimate the runoff potential using curve number method. Estimation of the same is carried out to determine and forecast its effects. Estimation of direct rainfall-runoff is always efficient (Bansode et al 2014).

MATERIAL AND METHODS

Study area: The Hebbur micro-watershed of Ajjampura sub-watershed, Tarikere taluk, Chikkamagalore district has been selected for analysis of runoff using SCS-CN Method. Hebbur Micro-Watershed is located between 13° 46' 14.81" N to 13° 44' 23.95" N Latitude and 75° 58' 24.38" E to 76° 0' 41.14" E Longitude, covering an area of 1038 ha (Fig. 1). Area falls under semi-arid region with average annual rainfall of 874 mm. Apart from rainfall, tanks and bore wells are the source of water resources. Watershed area is covered mostly of black soils however it also contains smaller portion of red and sandy textured soil. The major crops grown in study area are maize, ragi, onion, ginger, potato and chilly during *Kharif*, whereas chick pea, jawar, safflower, wheat and horsegram during *Rabi* season. Major horticulture crops grown are arecanut, coconut, banana, mango and pomegranate.

Method: Topographic data from Survey of India (SOI) toposheet (57 C/2) of scale 1:50,000 was used to identify the study area. Daily rainfall data (2009-2018) from Ajjampura rain gauge station was collected from KSNDCM, Bangalore. The Indian Remote Sensing satellite with Linear Imaging Self Scanning sensors (IRS – LISS III) data of scale 1:50000 were collected from KRSAC, Bengaluru to use land use/ land cover of the study area (Fig. 2).

The methodology adopted in assessing the runoff potential of the study area is explained in the following steps.

The first step is to determine the hydrologic soil group (HSG) of the particular soil. Soil data was obtained from Land

Use Inventory information under KWDP-II, Sujal-III Watershed Development project, for making appropriate hydrological soil classification A, B, C & D (Fig. 3). All soils are classified in one of four different categories-ranked A-D on the basis of their runoff potential (Sahoo and Patra 2014).

Class A soils mostly having high infiltration rates even when thoroughly wetted and consisting chiefly of deep, well to excessively drained sands or gravels. These soils have high rate of water transmission. [Ex-deep sand, deep loess and aggregated silt]

Class B soils have moderately fine to moderately coarse textures and are considered to have moderate infiltration rates when completely wet [Ex- shallow loess, sandy loam, red loamy soil, red sandy loam and red sandy soil]

Class C soils have moderately fine to fine textures with slow infiltration rates.[Ex- clayey loam, shallow sandy loam, soils usually high in clay, mixed red and black soils]

Class D soils are primarily clay soils or soils with clay pan that

have slow infiltration rates.[Ex-heavy plastic clays certain saline soils and deep black soils]

Next step is to determine the five-day antecedent moisture condition of the particular soil from the daily precipitation record. It is an index for basin wetness. AMC (Antecedent Moisture Conditions) indicates the moisture content of soil at the beginning of the rainfall event under consideration. Three levels of AMC were documented by SCS are AMC I, AMC II and AMC III. The limits of these three AMC classes are based on rainfall magnitude of previous five days and season (dormant season and growing season). The values of curve number are determined based on the antecedent moisture conditions (Table 1).

The third step is to decide-on the basis of the land cover, the cultivation treatment, the hydrologic condition of the soil, and the hydrologic soil group of the particular soil-the actual runoff curve number to use in determining daily runoff from precipitation (SCS, 1972). SCS runoff CN for hydrologic soil cover complex under AMC II condition for the study area is given in Table 2. CN for AMC I and CN for AMC III were determined using equation (1) and (2).

Equation (1): $CN_I = CN_{II} / (2.281 - 0.01281 CN_{II})$ (1)

Equation (2): $CN_{III} = CN_{II} / (0.427 - 0.00573 CN_{II})$ (2)

Area weighted composite curve number for various conditions of land use and hydrologic soil conditions are computed as follows

$$CN = (CN_1 \times A_1) + (CN_2 \times A_2) + \dots + (CN_n \times A_n) / A \dots (3)$$

Where A₁, A₂, A₃, ..., A_n represent land use areas of having CN values CN₁, CN₂, CN₃, ..., CN_n respectively and A is the total area of the watershed.

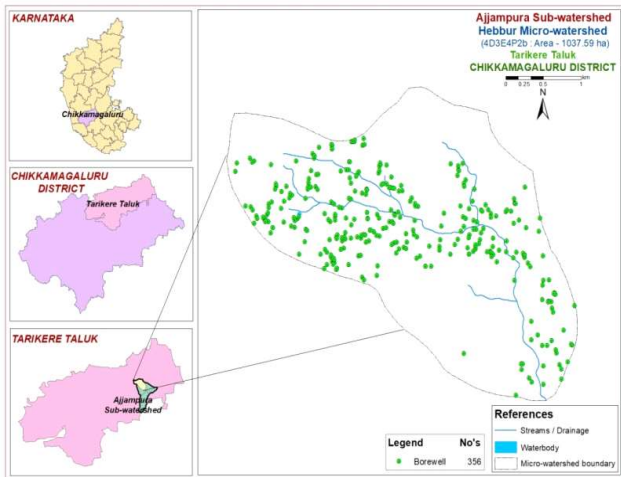


Fig. 1. Location map of the watershed

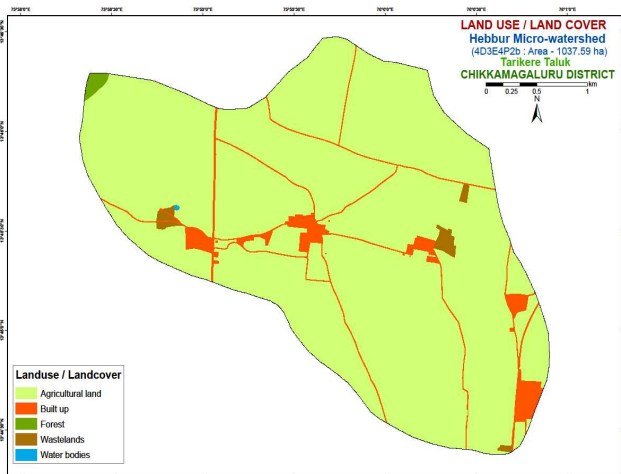


Fig. 2. Land use or land cover of the study area

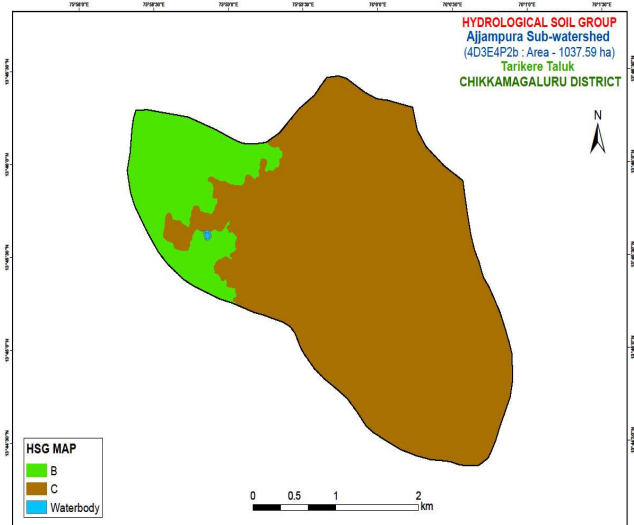


Fig. 3. HSG map of study area

Table 1. Antecedent moisture conditions for determining the value of CN

AMC type	Total rain in previous five days	
	Dormant season	Growing season
AMC-I: Soils are dry but not to wilting point. Satisfactory cultivation has taken place.	<13 mm	<36 mm
AMC-II: Average conditions	13-28 mm	36-53 mm
AMC-III: Sufficient rainfall has occurred within the immediate past 5 days. Saturated soil conditions prevail	>28 mm	>53 mm

Table 2. Curve number for HSG under AMC II conditions (Ahmad et al 2015)

Land use	Hydrological soil group			
	A	B	C	D
Agricultural land	76	86	90	93
Built up	49	69	79	84
Forest	26	40	58	61
Wastelands	71	80	85	88
Water bodies	97	97	97	97

Table 3. Sample of daily rainfall runoff computation of study area

Date	Rainfall (mm)	5 days cumulative rainfall (mm)	AMC condition	CN	S	Runoff (mm)
01-07-2009	0	11.5	1	78	71	0
02-07-2009	4	5	1	78	71	0
03-07-2009	9.5	14.5	2	89	31	0
04-07-2009	10.5	24	2	89	31	0
05-07-2009	4.5	28.5	3	95	13	0
06-07-2009	10.5	39	3	95	13	2
07-07-2009	2	37	3	95	13	0
08-07-2009	0.5	28	3	95	13	0
09-07-2009	3	20.5	2	89	31	0
10-07-2009	12.5	28.5	3	95	13	3
11-07-2009	0.5	18.5	2	89	31	0
12-07-2009	3.5	20	2	89	31	0
13-07-2009	3.5	23	2	89	31	0
14-07-2009	6.5	26.5	2	89	31	0
15-07-2009	32.5	46.5	3	95	13	19
16-07-2009	36.5	82.5	3	95	13	23
17-07-2009	19.5	98.5	3	95	13	8
18-07-2009	7.5	102.5	3	95	13	1
19-07-2009	7.5	103.5	3	95	13	1
20-07-2009	1	72	3	95	13	0
21-07-2009	1	36.5	3	95	13	0
22-07-2009	1	18	2	89	31	0
23-07-2009	1	11.5	1	78	71	0
24-07-2009	0	4	1	78	71	0
25-07-2009	0	3	1	78	71	0
26-07-2009	0	2	1	78	71	0
27-07-2009	1.5	2.5	1	78	71	0
28-07-2009	1	2.5	1	78	71	0
29-07-2009	2.5	5	1	78	71	0
30-07-2009	3.5	8.5	1	78	71	0
31-07-2009	0	8.5	1	78	71	0

Potential maximum retention is calculated using equation (4).

$$S = \left(\frac{24500}{CN} \right) - 254 \quad (4)$$

Runoff is estimated using equation (5).

$$Q = \left(\frac{(P - \lambda S)^2}{(P - (1 - \lambda)S)} \right) \quad (5)$$

For Indian conditions $\lambda = 0.3$

RESULTS AND DISCUSSION

The daily rainfall values were used as inputs to compute daily runoff. Using CN for AMC-II conditions, CN for AMC-I and AMC-III were calculated using equations (1)-(2). Using equation (3) weighted CN was calculated. Using equation (5) the daily runoff depth were computed. First daily rainfall runoff computation is shown in Table 4.

From the daily runoff, monthly and annual values can be derived. The annual runoff depths are computed for each rainfall event for the years 2009 - 2018 is shown in Table 5 and the relationship between rainfall-runoff is shown in Figure 4. Highest runoff was observed during 2014 which was about 42 percent of the annual rainfall followed by 2009 (39%) and 2010 (37%). Lowest was observed during 2016

Table 4. Runoff values (2009-2018)

Year	Rainfall (mm)	Runoff (mm)
2009	1051	404.74
2010	1167	440.08
2011	808	224.38
2012	513	152.60
2013	969	312.92
2014	1239	520.14
2015	1087	378.89
2016	542	107.08
2017	663	212.30
2018	1011	328.02

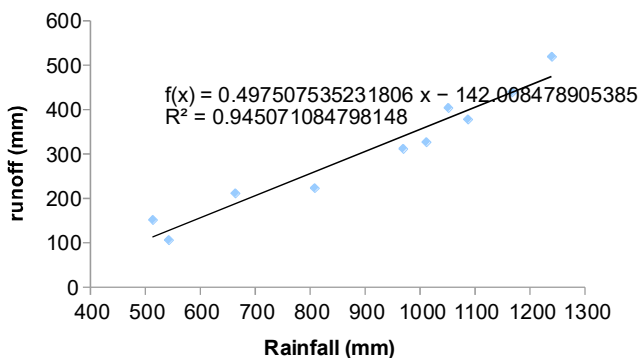


Fig. 4. Rainfall-runoff relationship

due to low rainfall. Increase in runoff was observed with increase in rainfall in the year. Coefficient of determination for rainfall runoff was 0.94 for the watershed (Fig. 4).

CONCLUSION

Daily runoff was estimated between 2009-2018. Highest runoff was observed during 2014 (520 mm) followed by 2010 and 2009. During 2016 lowest runoff was generated in the watershed due to low rainfall in the year. Correlation coefficient between rainfall and runoff was found 97 per cent between 2009-2018. This estimated runoff will be helpful for planning of the soil and water conservation structures in the watershed.

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